

C5.2 Elasticity and Plasticity

Lecture 4 — Antiplane strain and torsion

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Antiplane strain

- ▶ Consider displacement field

$$\mathbf{u} = \begin{pmatrix} 0 \\ 0 \\ w(x, y) \end{pmatrix}$$

- ▶ The stress tensor is given by

$$\mathcal{T} = \begin{pmatrix} 0 & 0 & \tau_{xz} \\ 0 & 0 & \tau_{yz} \\ \tau_{xz} & \tau_{yz} & 0 \end{pmatrix}$$

where

$$\tau_{xz} = \mu \frac{\partial w}{\partial x}, \quad \tau_{yz} = \mu \frac{\partial w}{\partial y}$$

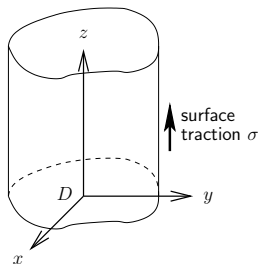
- ▶ Steady Navier eqn (no body force) \rightarrow Laplace's equation

$$\frac{\partial \tau_{xz}}{\partial x} + \frac{\partial \tau_{yz}}{\partial y} = 0 \quad \Rightarrow \quad \frac{\partial^2 w}{\partial x^2} + \frac{\partial^2 w}{\partial y^2} = 0$$

Antiplane strain

Can be created by applying shear stress σ to boundary of a cylindrical bar.

$$\begin{pmatrix} 0 & 0 & \tau_{xz} \\ 0 & 0 & \tau_{yz} \\ \tau_{xz} & \tau_{yz} & 0 \end{pmatrix} \begin{pmatrix} n_x \\ n_y \\ 0 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ \sigma \end{pmatrix} \text{ on } \partial D$$



- ▶ Leads to the **Neumann problem**

$$\nabla^2 w = 0 \text{ in } D, \quad \mu \frac{\partial w}{\partial n} = \sigma \text{ on } \partial D.$$

- ▶ Note that the elastic energy in antiplane strain is given by

$$\mathcal{U}[w] = \iint_D \mu |\nabla w|^2 \, dx dy$$

Min $\mathcal{U}[w]$ via calculus of variations \rightarrow Laplace equation $\nabla^2 w = 0$.

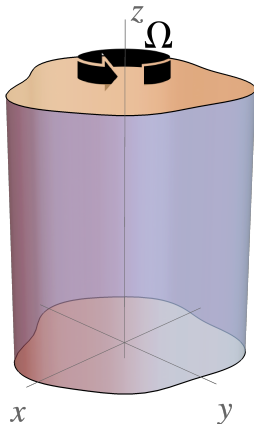
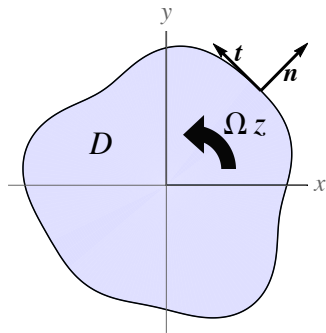
Torsion

Displacement

$$\mathbf{u} = \begin{pmatrix} -\Omega y z \\ \Omega x z \\ \Omega \psi(x, y) \end{pmatrix}$$

describes a bar being
twisted about the z -axis

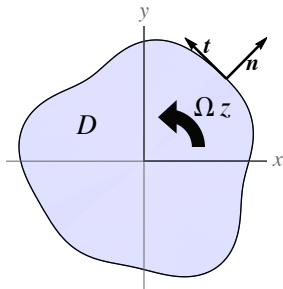
In-plane displacement $(u, v) = \Omega z(-y, x)$



Torsion

With $\mathbf{u} = \begin{pmatrix} -\Omega yz \\ \Omega xz \\ \Omega\psi(x, y) \end{pmatrix}$ only nonzero stress components are...

$$\tau_{xz} = \mu\Omega \left(\frac{\partial\psi}{\partial x} - y \right), \quad \tau_{yz} = \mu\Omega \left(\frac{\partial\psi}{\partial y} + x \right).$$



Navier eqn and stress-free BC:

$$\frac{\partial\tau_{xz}}{\partial x} + \frac{\partial\tau_{yz}}{\partial y} = 0 \quad \text{in } D$$

$$\tau_{xz}n_x + \tau_{yz}n_y = 0 \quad \text{on } \partial D$$

Tangent and normal vectors given by

$$\mathbf{t} = \begin{pmatrix} x'(s) \\ y'(s) \\ 0 \end{pmatrix}, \quad \mathbf{n} = \begin{pmatrix} n_x \\ n_y \\ 0 \end{pmatrix} = \begin{pmatrix} y'(s) \\ -x'(s) \\ 0 \end{pmatrix}, \quad s = \text{arc-length.}$$

Torsion

We end up with **well posed BVP** for $\psi(x, y)$ (up to a constant):

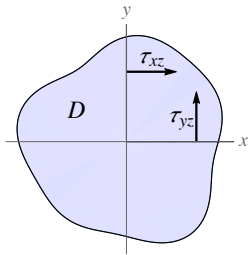
$$\nabla^2 \psi = 0 \quad \text{in } D$$

$$\frac{\partial \psi}{\partial n} = \frac{1}{2} \frac{d}{ds} (x^2 + y^2) \quad \text{on } \partial D$$

(NB solvability condition satisfied identically)

Given shape of cross-section D , solve for ψ .
Then calculate **torque** M using

$$M = \iint_D (x\tau_{yz} - y\tau_{xz}) \, dx dy$$



Thus find that $M = R\Omega$ where **torsional rigidity**

$$R = \mu \iint_D \left(x \frac{\partial \psi}{\partial y} - y \frac{\partial \psi}{\partial x} + x^2 + y^2 \right) \, dx dy$$

Example — a circular bar

- ▶ Suppose cross-section D is a disc of radius a .
- ▶ Then ψ satisfies [in polar coords (r, θ)]

$$\nabla^2 \psi = 0 \quad \text{in } r < a, \quad \frac{\partial \psi}{\partial r} = \frac{1}{2} \frac{d}{ds} (x^2 + y^2) = 0 \quad \text{on } r = a$$

- ▶ Solution is $\psi = \text{constant}$
- ▶ Torsional rigidity

$$\begin{aligned} R &= \mu \iint_D \left(x \frac{\partial \psi}{\partial y} - y \frac{\partial \psi}{\partial x} + x^2 + y^2 \right) dx dy \\ &= \mu \int_0^{2\pi} \int_0^a r^3 dr d\theta \end{aligned}$$

$$\Rightarrow \quad R = \frac{\pi a^4 \mu}{2} \quad \text{torsional rigidity of a circular bar}$$

Stress function

- ▶ Problem can be reformulated in terms of a **stress function**

- ▶ Whenever stress tensor is of the form $\mathcal{T} = \begin{pmatrix} 0 & 0 & \tau_{xz} \\ 0 & 0 & \tau_{yz} \\ \tau_{xz} & \tau_{yz} & 0 \end{pmatrix}$,

with $\tau_{xz} = \tau_{xz}(x, y)$ and $\tau_{yz} = \tau_{yz}(x, y) \dots$

- ▶ Navier equation reduces to $\frac{\partial \tau_{xz}}{\partial x} + \frac{\partial \tau_{yz}}{\partial y} = 0$

- ▶ We deduce the existence of a **stress function** $\phi(x, y)$ such that

$$\mu\Omega \left(\frac{\partial \psi}{\partial x} - y \right) = \tau_{xz} = \mu\Omega \frac{\partial \phi}{\partial y}$$
$$\mu\Omega \left(\frac{\partial \psi}{\partial y} + x \right) = \tau_{yz} = -\mu\Omega \frac{\partial \phi}{\partial x}$$

(factor of $\mu\Omega$ included for convenience)

Stress function

- ▶ Functions $\phi(x, y)$ and $\psi(x, y)$ satisfy the relations

$$\begin{aligned}\frac{\partial \psi}{\partial x} &= \frac{\partial \phi}{\partial y} + y & \frac{\partial \psi}{\partial y} &= -\frac{\partial \phi}{\partial x} - x \\ &= \frac{\partial}{\partial y} \left[\phi + \frac{1}{2} (x^2 + y^2) \right] & &= -\frac{\partial}{\partial x} \left[\phi + \frac{1}{2} (x^2 + y^2) \right]\end{aligned}$$

- ▶ ψ and $\phi + \frac{1}{2} (x^2 + y^2)$ are **harmonic conjugates**
- ▶ Eliminate ψ — stress function satisfies **Poisson's equation**

$$\boxed{\nabla^2 \phi = -2} \quad \text{in cross-section } D.$$

- ▶ Boundary condition $\mathcal{T}\mathbf{n} = \mathbf{0}$ on ∂D :

$$\frac{d\phi}{ds} = \frac{\partial \phi}{\partial y} y'(s) + \frac{\partial \phi}{\partial x} x'(s) = 0$$

- ▶ i.e. $\phi = \text{constant}$ on ∂D and wlog $\boxed{\phi = 0 \text{ on } \partial D}$

Stress function

Now calculate **torsional rigidity** R :

$$\begin{aligned} R &= \mu \iint_D \left(x \frac{\partial \psi}{\partial y} - y \frac{\partial \psi}{\partial x} + x^2 + y^2 \right) dx dy \\ &= -\mu \iint_D \left(x \frac{\partial \phi}{\partial x} + y \frac{\partial \phi}{\partial y} \right) dx dy \end{aligned}$$

After using Divergence Theorem and BCs...

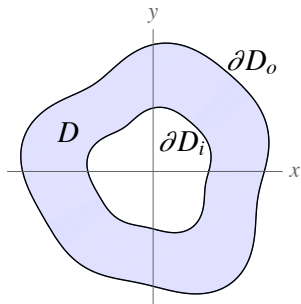
$$R = 2\mu \iint_D \phi dx dy$$

Calculation with ϕ looks more convenient than ψ

Exercise: reproduce calculation of R for a circular bar using ϕ

Multiply connected domains

Most torsion bars are actually **tubes** (e.g. bike frame)



- ▶ Zero stress on BCs
 $\Rightarrow \phi = \text{constant}$ on ∂D_o and ∂D_i
- ▶ We can only set one constant to zero without loss of generality!

So we have to solve

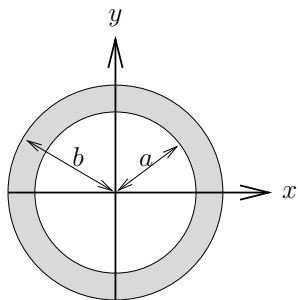
$$\begin{aligned}\nabla^2 \phi &= -2 && \text{in } D \\ \phi &= 0 && \text{on } \partial D_o \\ \phi &= k && \text{on } \partial D_i\end{aligned}$$

- ▶ k is **to be determined**
- ▶ If there are many holes then there are many undetermined constants k_i !
- ▶ Torsional rigidity [**exercise**]

$$R = 2\mu \iint_D \phi \, dx dy + 2\mu k A_i$$

- ▶ A_i is **area of hole**

Example — a circular cylindrical bar



Stress function $\phi(r)$ satisfies

$$\frac{1}{r} \frac{d}{dr} \left(r \frac{d\phi}{dr} \right) = -2 \quad a < r < b$$

$$\phi = 0 \quad r = b$$

$$\phi = k \quad r = a$$

Solution...
$$\phi(r) = \frac{b^2 - r^2}{2} + \left(k - \frac{b^2 - a^2}{2} \right) \frac{\log(r/b)}{\log(a/b)}$$

► But how to determine k ?

Multiply connected domains

- ▶ To close the problem we must ensure that **displacement** $w = \Omega\psi$ is **single-valued** as we go around the hole...

$$0 = [\psi]_{\partial D_i} = \int_{\partial D_i} \frac{d\psi}{ds} ds = \dots$$

$$\Rightarrow \boxed{\oint_{\partial D_i} \frac{\partial \phi}{\partial n} ds = -2A_i}$$

- ▶ Extra condition ensures w is well defined and determines unknown k .

Example — a circular cylindrical bar

- ▶ For circular tube $a < r < b$,

$$\phi(r) = \frac{b^2 - r^2}{2} + \left(k - \frac{b^2 - a^2}{2} \right) \frac{\log(r/b)}{\log(a/b)}$$

- ▶ The closure condition $\oint_{\partial D_i} \frac{\partial \phi}{\partial n} ds = -2A_i$

$$\Rightarrow \int_0^{2\pi} \left. \frac{d\phi}{dr} \right|_{r=a} a d\theta = -2\pi a^2$$

- ▶ We find $k = \frac{b^2 - a^2}{2}$
- ▶ NB this eliminates the $\log r$ in ϕ corresponding to θ in ψ .
- ▶ Then torsional rigidity is given by $R = \frac{\mu\pi}{2} (b^4 - a^4)$

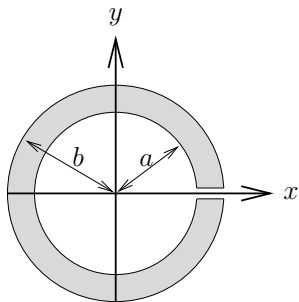
Rigidity of a thin tube

- ▶ For a tube $a < r < b$ we have $R = \frac{\mu\pi}{2} (b^4 - a^4)$
- ▶ In practice tubes usually have thin walls.
- ▶ With $b = a(1 + \epsilon)$ with $\epsilon \ll 1$ we get

$$R \sim 2\mu\pi a^4 \epsilon$$

- ▶ Now compare with a thin **cut** tube (e.g. rusted through)...

Rigidity of a thin cut tube



Stress function ϕ satisfies

$$\nabla^2 \phi = -2 \quad a < r < b, \quad 0 < \theta < 2\pi$$

$$\phi = 0 \quad r = a, \quad r = b$$

$$\phi = 0 \quad \theta = 0, \quad \theta = 2\pi$$

i.e. $k = 0$

- ▶ Assume wall is thin so $b = a(1 + \epsilon)$ with $\epsilon \ll 1$.
- ▶ Let $r = a(1 + \epsilon\xi)$ so $\xi \in [0, 1]$ and...

$$\nabla^2 \phi \sim \frac{1}{\epsilon^2 a^2} \frac{\partial^2 \phi}{\partial \xi^2} \sim -2$$

with $\phi = 0$ at $\xi = 0, 1$

Rigidity of a thin cut tube

Leading-order solution $\phi \sim \epsilon^2 a^2 \xi(1 - \xi)$ and hence torsional rigidity...

$$R = 2\mu \int_0^{2\pi} \int_a^{a(1+\epsilon)} \phi r \, dr d\theta \sim 4\pi\mu \int_0^1 \epsilon^2 a^2 \xi(1 - \xi) \epsilon a^2 \, d\xi$$

i.e.

$$R = \frac{2\pi}{3} \mu a^4 \epsilon^3$$

- ▶ *cf* $R \sim 2\pi\mu a^4 \epsilon$ for pristine tube.
- ▶ Small cut reduces R by two orders of magnitude!