C5.2 Elasticity and Plasticity

Lecture 7 — Models for thin structures

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Small oscillations of a string

A thin string undergoes small transverse displacement w(x,t)



▶ Apply Newton's 2^{nd} law to a small segment $[x, x + \delta x]$

$$\rho \delta x \frac{\partial^2 w}{\partial t^2} \begin{pmatrix} 0\\ 1 \end{pmatrix} = \left[T \begin{pmatrix} \cos \theta\\ \sin \theta \end{pmatrix} \right]_x^{x+\delta x} + \rho g \delta x \begin{pmatrix} 0\\ -1 \end{pmatrix}$$

Small oscillations of a string

$$\rho \delta x \frac{\partial^2 w}{\partial t^2} \begin{pmatrix} 0\\ 1 \end{pmatrix} = \left[T \begin{pmatrix} \cos \theta\\ \sin \theta \end{pmatrix} \right]_x^{x+\delta x} + \rho g \delta x \begin{pmatrix} 0\\ -1 \end{pmatrix}$$

For small θ we have $\cos \theta \sim 1$ and $\sin \theta \sim \theta \sim \frac{\partial w}{\partial x}$

• Let
$$\delta x \to 0$$
 to get...

$$\frac{\partial T}{\partial x} = 0 \qquad \qquad \rho \frac{\partial^2 w}{\partial t^2} = T \frac{\partial^2 w}{\partial x^2} - \rho g$$

- Tension T only depends on t (usually constant)
- ▶ For *T* > 0, *w* satisfies the (forced) wave equation
- For T < 0 PDE changes type to elliptic and becomes ill posed
- String can't withstand any compressive force : it has no stiffness

Small transverse displacement of a beam

A beam can withstand significant transverse shear force N and bending moment M (about y-axis).



Forces and moments acting on a segment $[x, x + \delta x]$ of a beam

NB define N to act in transverse not normal direction

First incorporate N in force balance (Problem sheet 2):

$$\frac{\partial T}{\partial x} = 0 \qquad \qquad \rho \frac{\partial^2 w}{\partial t^2} = T \frac{\partial^2 w}{\partial x^2} + \frac{\partial N}{\partial x} - \rho g$$

Next perform moment balance

$$N = \frac{\partial M}{\partial x}$$

The beam equation

$$\rho \frac{\partial^2 w}{\partial t^2} = T \frac{\partial^2 w}{\partial x^2} + \frac{\partial N}{\partial x} - \rho g \qquad \qquad N = \frac{\partial M}{\partial x}$$

- ▶ To close the problem need a constitutive relation for *M*.

$$M = -B \frac{\partial^2 w}{\partial x^2}$$

$$B = EI =$$
bending stiffness

- Assume this relation holds for all beam deformations.
- ▶ Then *w* satisfies the (linear) beam equation

$$\rho \frac{\partial^2 w}{\partial t^2} = T \frac{\partial^2 w}{\partial x^2} - B \frac{\partial^4 w}{\partial x^4} - \rho g$$

▶ 4th order PDE — needs 2 BCs at each end of the beam...

Boundary conditions

Possible boundary conditions include...



$$w = \frac{\partial w}{\partial x} = 0$$

(b) Simple support
$$w = M = 0$$
 $w = \frac{\partial^2 w}{\partial x^2} = 0$

(c) Free
$$M = N = 0$$

$$\frac{\partial^2 w}{\partial x^2} = \frac{\partial^3 w}{\partial x^3} = 0$$

Example — steady compression of a beam

- ▶ Unlike string, beam can withstand compressive stress, T < 0
- Consider steady beam of length L with no body force and T = −P < 0:</p>

$$Bw^{\prime\prime\prime\prime}(x) + Pw^{\prime\prime}(x) = 0$$

Example boundary conditions: clamped horizontal but zero transverse force (w' = N = 0)

$$w^\prime(x)=w^{\prime\prime\prime}(x)=0 \text{ at } x=0,\,L$$

Eigenvalue problem: w = const is always a solution. Non-trivial solutions exist...

$$w(x) = A\cos\left(\frac{n\pi x}{L}\right) \quad \Longleftrightarrow \quad \frac{P}{B} = \frac{n^2\pi^2}{L^2}$$
$$(n = 1, 2, \cdots)$$

Compression of a beam



- At each critical load, beam can buckle with increasingly oscillatory buckling modes and amplitude A is indeterminate.
- When not at critical load, amplitude must be zero.
- Problem sheet 2 \Rightarrow zero solution is unstable for $PL^2/B > \pi^2$.
- To understand buckling behaviour properly we need weakly nonlinear theory...