

### Problem Sheet 3

1. An elastic beam with bending stiffness  $EI$  is in equilibrium subject to a compressive force  $P_0$  and shear force  $N_0$  applied at its ends, where it is clamped parallel to the  $x$ -axis. Show that, if the beam makes an angle  $\theta(s)$  with the  $x$ -axis, where  $s$  is arc-length, the shear force  $N$  and bending moment  $M$  at any point satisfy

$$N = N_0 \cos \theta + P_0 \sin \theta, \quad \frac{dM}{ds} - N = 0.$$

Assuming the constitutive relation  $M = -EI d\theta/ds$ , obtain the *Euler strut* equation

$$EI \frac{d^2\theta}{ds^2} + P_0 \sin \theta + N_0 \cos \theta = 0.$$

- (a) When the applied shear force is zero, obtain the dimensionless model

$$\frac{d^2\theta}{d\xi^2} + \pi^2 \lambda \sin \theta = 0, \quad \theta(0) = \theta(1) = 0,$$

where the dimensionless variable  $\xi$  and parameter  $\lambda$  are to be defined.

- (b) Assuming  $|\theta| \ll 1$ , show that nontrivial solutions  $\theta = A \sin(n\pi\xi)$  exist when  $\lambda = n^2$ , where  $n$  is a positive integer.
- (c) Now suppose that  $\lambda$  is close to one of the critical values so that  $\lambda = n^2 + \varepsilon\lambda_1$ , where  $0 < \varepsilon \ll 1$ . Show that solutions of the form

$$\theta = \varepsilon^{1/2} \{A_0 \sin(n\pi\xi) + \varepsilon\Theta_1 + O(\varepsilon^2)\}$$

exist provided the leading-order amplitude  $A_0$  satisfies

$$A_0 \left( A_0^2 - \frac{8\lambda_1}{n^2} \right) = 0.$$

Plot the resulting response diagram.

- (d) Now suppose there is a *small* applied shear force  $N_0$ , so that

$$\frac{d^2\theta}{d\xi^2} + \pi^2 \lambda \sin \theta + \varepsilon^{3/2} F \cos \theta = 0.$$

Define  $F$  in terms of  $N_0$ . Repeat the analysis of part (c) with  $n = 1$  to show that  $A_0$  now satisfies

$$A_0 (A_0^2 - 8\lambda_1) = \frac{32F}{\pi^3}.$$

Sketch the response diagram. Assuming that  $F > 0$ , show that a negative amplitude  $A_0$  is possible only if the forcing parameter  $\lambda_1$  exceeds  $3F^{2/3}/2^{1/3}\pi^2$ .

2. (a) An elastic string is stretched to a uniform tension  $T$  over a nearly flat obstacle  $z = f(x)$ . If a transverse body force  $p(x)$  per unit length is applied, show that the transverse displacement  $z = w(x)$  satisfies  $Td^2w/dx^2 = p(x)$  in the non-contact set and  $w = f$  in the contact set, with continuity of  $w$  and  $dw/dx$  on the boundary between them.
- (b) Show that the above model is not complete by finding *three* solutions when  $f(x) = -7/500$ ,  $p(x)/T = x^2 - 4/75$  and  $w = 0$  at  $x = \pm 1$ .
- (c) Which solution from part (b) satisfies the *complementarity conditions*

$$(w - f) \left( p - T \frac{d^2w}{dx^2} \right) = 0, \quad w - f \geq 0, \quad p - T \frac{d^2w}{dx^2} \geq 0? \quad (*)$$

Interpret these conditions physically.

3. It may be shown that  $(*)$  is equivalent to the *variational inequality*

$$T \int_{-1}^1 \frac{dw}{dx} \left( \frac{dv}{dx} - \frac{dw}{dx} \right) dx \geq \int_{-1}^1 p(w - v) dx \quad \text{for all } v \geq f. \quad (\dagger)$$

Now we will show that  $(\dagger)$  is equivalent to minimising the net strain and potential energy over all displacements that do not interpenetrate the obstacle.

- (a) Show that, if

$$U[w] = \int_{-1}^1 \left( \frac{T}{2} \left( \frac{dw}{dx} \right)^2 + pw \right) dx,$$

then

$$U[w] - U[v] = \int_{-1}^1 p(w - v) dx - T \int_{-1}^1 \frac{dw}{dx} \left( \frac{dv}{dx} - \frac{dw}{dx} \right) dx - \frac{T}{2} \int_{-1}^1 \left( \frac{dw}{dx} - \frac{dv}{dx} \right)^2 dx$$

and deduce that, if  $w$  satisfies  $(\dagger)$ , then it minimises  $U$ .

- (b) Note that, if  $v_1$  and  $v_2$  belong to the set  $\{v : v \geq f \text{ on } (-1, 1)\}$ , then so does  $\alpha v_1 + (1 - \alpha)v_2$  for  $0 < \alpha < 1$  [*this means that the set is convex*]. Show that if  $w$  minimises  $U$ , then

$$U[w] \leq U[\alpha v + (1 - \alpha)w]$$

for all  $v \geq f$ . Expand this inequality for small  $\alpha$  to obtain  $(\dagger)$ .

4. A thin elliptical Mode III crack, whose boundary  $\partial\Omega$  is given by

$$\frac{x^2}{c^2 \cosh^2 \varepsilon} + \frac{y^2}{c^2 \sinh^2 \varepsilon} = 1,$$

is subject to an antiplane strain displacement field  $\mathbf{u} = (0, 0, w(x, y))^T$ .

(a) If a shear stress  $\tau_{yz} = \sigma$  is applied in the far field, justify the conditions

$$\frac{\partial w}{\partial n} = 0 \quad \text{on } \partial\Omega, \quad w \sim \frac{\sigma y}{\mu} \quad \text{as } x^2 + y^2 \rightarrow \infty.$$

(b) Show that the *Joukowski transformation*

$$x + iy = z = \frac{c}{2} \left( \zeta + \frac{1}{\zeta} \right)$$

conformally maps the region  $|\zeta| > e^\varepsilon$  ( $\varepsilon > 0$ ) onto the outside of the crack. What happens as  $\varepsilon \rightarrow 0$ ? What is the inverse map from  $z$  to  $\zeta$ ?

(c) Introducing polar coordinates  $(r, \theta)$  such that  $\zeta = re^{i\theta}$ , show that  $w$  satisfies the conditions

$$\frac{\partial w}{\partial r} = 0 \quad \text{on } r = e^\varepsilon, \quad w \sim \frac{c\sigma}{2\mu} r \sin \theta \quad \text{as } r \rightarrow \infty.$$

Hence obtain the solution

$$w = \frac{c\sigma}{2\mu} \operatorname{Im} \left\{ \zeta - \frac{e^{2\varepsilon}}{\zeta} \right\}.$$

(d) In the limit  $\varepsilon \rightarrow 0$ , deduce that

$$w = \frac{\sigma}{\mu} \operatorname{Im} \left\{ \sqrt{z^2 - c^2} \right\}, \quad (\ddagger)$$

and carefully define the square root.

5. If the displacement in antiplane strain is given by  $w(x, y) = \operatorname{Im} \{f(z)\}$ , where  $z = x + iy$ , show that the corresponding stress components are

$$\tau_{xz} = \mu \operatorname{Im} \{f'(z)\}, \quad \tau_{yz} = \mu \operatorname{Re} \{f'(z)\}.$$

Hence show that the stress components ahead of the crack, on  $y = 0$ ,  $x > c$ , due to the displacement field  $(\ddagger)$ , are given by

$$\tau_{xz} = 0, \quad \tau_{yz} = \frac{\sigma x}{\sqrt{x^2 - c^2}}.$$

Suppose that the crack tip propagates when the *stress intensity factor*

$$K_{\text{III}} = \sqrt{2\pi} \lim_{x \downarrow c} \{ \tau_{yz}(x, 0) \sqrt{x - c} \}$$

exceeds a critical value  $K_*$ . Deduce that the crack will grow if the applied shear stress exceeds  $K_*/\sqrt{\pi c}$ .